

FEM/FEA Boundary Conditions, and Failure Analysis

Three Cases must use FEM/FEA:

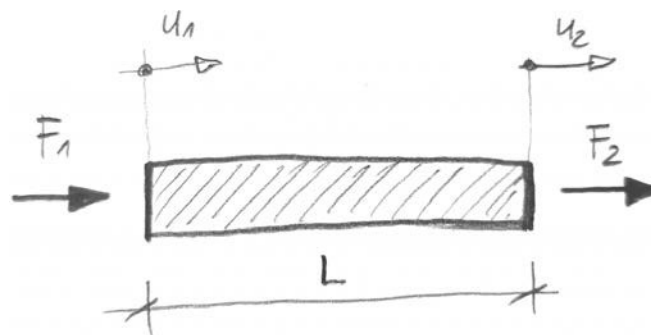
- (1) Irregular Geometry
- (2) Complex Material Properties
- (3) Nonlinearity

NOTE: Three general cases you would need to use FEA for your Analysis and Analytical Solutions do not apply:

- (1) Irregular Geometry,
- (2) Complex and Non-homogenous Materials Properties, and
- (3) Nonlinearity of the Problems

I: Classical 1-D FEA Problem (Axial Loading of a Bar)

<https://enterfea.com/finite-element-analysis-by-hand/>



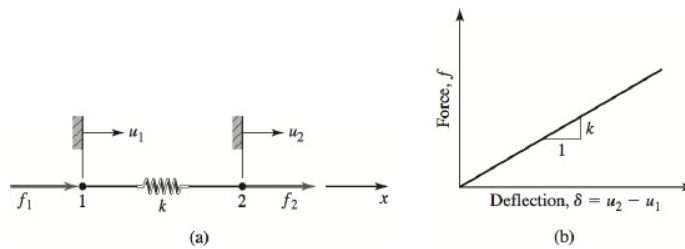
$$G = E \cdot \epsilon$$

STRESS: $G = \frac{F}{A}$ (FORCE / CROSS-SECTION AREA)
 YOUNG'S MODULUS
 STRAIN: $\epsilon = \frac{\Delta L}{L}$ (CHANGE IN LENGTH / ORIGINAL LENGTH)
 $\Delta L = u_2 - u_1$

$$\frac{F}{A} = G = E \cdot \epsilon = E \cdot \frac{\Delta L}{L} = E \cdot \frac{u_2 - u_1}{L}$$

$$F = \frac{A \cdot E}{L} (u_2 - u_1)$$

$$\mathbf{F} = \mathbf{K} \Delta \mathbf{X} \text{ , where } \mathbf{K} = \frac{A \cdot E}{L} \text{ \& } \Delta \mathbf{X} = \Delta U$$



$$F_1 = -\frac{A \cdot E}{L} (u_2 - u_1) = \frac{A \cdot E}{L} (u_1 - u_2)$$

$$F_2 = \frac{A \cdot E}{L} (u_2 - u_1)$$

$$\begin{array}{ccc}
 \text{FORCES} & \text{STIFFNESS} & \text{DISPLACEMENTS} \\
 \text{VECTOR} & \text{MATRIX} & \text{VECTOR} \\
 \downarrow & \downarrow & \downarrow \\
 \begin{bmatrix} F_1 \\ F_2 \end{bmatrix} = \begin{bmatrix} \frac{A \cdot E}{L} & -\frac{A \cdot E}{L} \\ -\frac{A \cdot E}{L} & \frac{A \cdot E}{L} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix}
 \end{array}$$

$$AE/L = K$$

$$\begin{bmatrix} k & -k \\ -k & k \end{bmatrix} \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} = \begin{Bmatrix} f_1 \\ f_2 \end{Bmatrix}$$

$$\begin{Bmatrix} f_1 \\ f_2 \end{Bmatrix} = [k_e] \begin{Bmatrix} u_1 \\ u_2 \end{Bmatrix} \quad \text{with} \quad [k_e] = \begin{bmatrix} k & -k \\ -k & k \end{bmatrix}$$

$\downarrow \qquad \qquad \downarrow \qquad \qquad \downarrow$
 known $\{F\} = [K] \{X\}$ unknown

$$\begin{array}{ccc}
 \{F\} = [K] \cdot \{U\} \\
 \text{Array of applied forces} & & \text{Array of displacements (one for each DOF)} \\
 \text{(one for each DOF)} & & \\
 | & & \\
 \text{Matrix of} & & \\
 \text{stiffnesses} & & \\
 \text{(DOF x DOF)} & &
 \end{array}$$

Pre-Dot the above Eq by $[K]^{-1}$

$$[K]^{-1} \{F\} = [K]^{-1} [K] \{U\}, \text{ where } [K]^{-1} [K] = [1] \implies [K]^{-1} \{F\} = \{U\}$$

$$\{U_i\} = [K_{ij}]^{-1} \{F_j\}$$

$$\{\epsilon_i\} = [d/dx_{ij}] \{u_j\}, \quad \{\sigma_i\}_{6 \times 1} = [C_{ij}]_{6 \times 6} \{\epsilon_i\}_{6 \times 1}$$

General 3D Case:

$$\sigma_i = \sigma_x, \sigma_y, \sigma_z, \sigma_{xy}, \sigma_{yz}, \sigma_{zx} \text{ \& } \sigma_{ij} = \sigma_{ji}$$

$$\epsilon_i = \epsilon_x, \epsilon_y, \epsilon_z, \epsilon_{xy}, \epsilon_{yz}, \epsilon_{zx} \text{ \& } \epsilon_{ij} = \epsilon_{ji}$$

II: 3D General Materials Properties in Mechanics

$C_{ij} = C_{ji} = 21$ independent materials Anisotropic properties $[C_{ij}]$

= **9** independent materials Orthotropic properties (3E's, 3Gs, 3Vs)

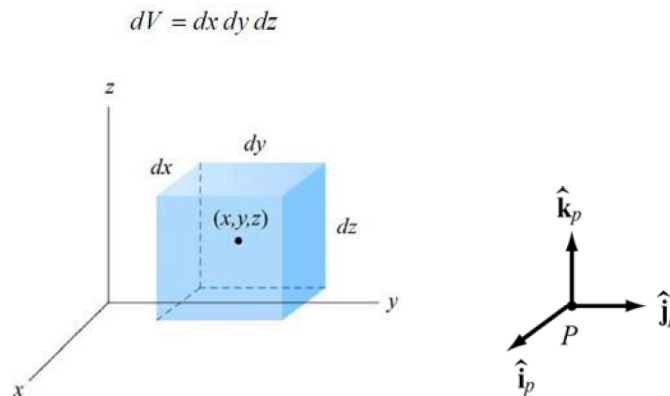
= **5** independent materials Transversely-Isotropic properties (2Es, 1G, 2Vs)

= **2** independent materials Isotropic properties (E & V, since $G = E/(2(1+V))$)

III. Geometries

For the following Regular Geometries Analytical Solutions and associated **Governing Equations** (written for the given coordinate system) plus the boundary and initial conditions can be used. Otherwise (i.e., Irregular Geometries) FEA should be used).

(III.1): Cartesian/Orthogonal Coordinate System: x, y, z



Volume element in Cartesian coordinates

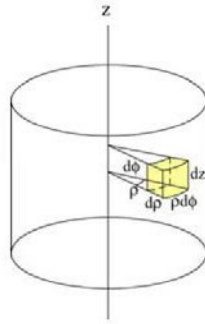
Unit Vectors

Heat Conduction Eq in Cartesian Coordinate System:

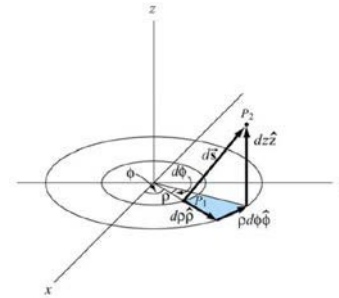
$$\frac{\partial^2 T}{\partial x^2} + \frac{\dot{e}_{\text{gen}}}{k} = \frac{1}{\alpha} \frac{\partial T}{\partial t}$$

(III.2): Cylindrical/Orthogonal Coordinate System: ρ, ϕ, z

$$dV = \rho d\phi d\rho dz$$



Volume element for a cylinder



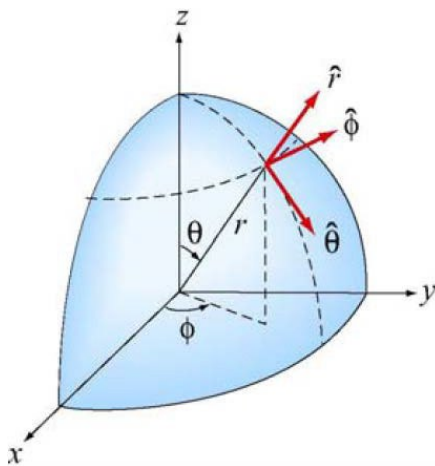
Unit Vectors

Heat Conduction Eq in Cylindrical Coordinate System:

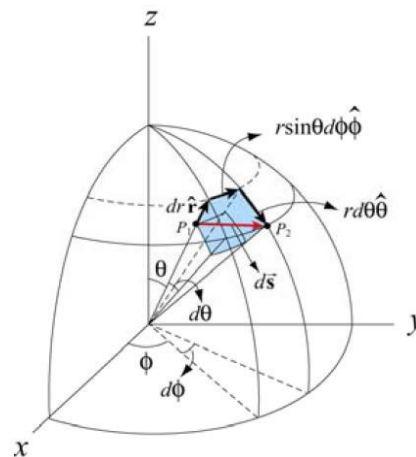
$$\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial T}{\partial r} \right) + \frac{\dot{e}_{\text{gen}}}{k} = \frac{1}{\alpha} \frac{\partial T}{\partial t}$$

(III.3): Spherical/Orthogonal Coordinate System: r, ϕ, θ

$$dV = r^2 \sin \theta d\theta d\phi dr$$



Spherical coordinates



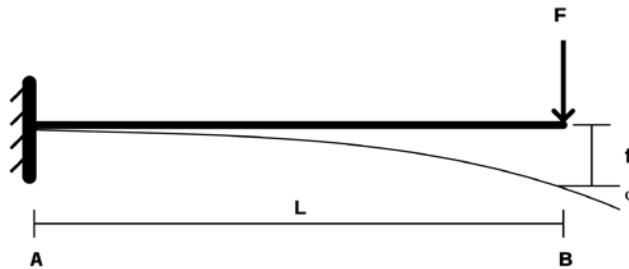
Unit Vectors

Heat Conduction Eq in Spherical Coordinate System:

$$\frac{1}{r^2} \frac{\partial}{\partial r} \left(r^2 \frac{\partial T}{\partial r} \right) + \frac{\dot{e}_{\text{gen}}}{k} = \frac{1}{\alpha} \frac{\partial T}{\partial t}$$

IV. Nonlinearity:

IV.1. Nonlinearity due to Small Deformation versus Large Deformation



If “ f ” is the tip Deflection and “ t ” is the thickness of a beam, then:

If $f \ll t$, then f is considered “small deformation” and the problem is Linear.

If $f \gg t$, then f is considered “large deformation” and the problem is Nonlinear.

IV.2. Nonlinearity due to the Governing Equations

If the Field/Dependent Variable within the Individual Terms of a Governing Equation is to the first power only, then the problem is Linear.

If the Field/Dependent Variable within the Individual Terms of a Governing Equation is to the second or higher power, then the problem is Nonlinear.

If K is variable in space, the 3D Heat Conduction Eq is:

$$\frac{\partial}{\partial x} \left(k \frac{\partial T}{\partial x} \right) + \frac{\partial}{\partial y} \left(k \frac{\partial T}{\partial y} \right) + \frac{\partial}{\partial z} \left(k \frac{\partial T}{\partial z} \right) + \dot{e}_{\text{gen}} = \rho c \frac{\partial T}{\partial t}$$

If K is constant in space and the material is **Isotropic**, the 3D Heat Conduction Eq is (where, $\alpha = k/\rho c$: Thermal Diffusivity)):

$$\frac{\partial^2 T}{\partial x^2} + \frac{\partial^2 T}{\partial y^2} + \frac{\partial^2 T}{\partial z^2} + \frac{\dot{e}_{\text{gen}}}{k} = \frac{1}{\alpha} \frac{\partial T}{\partial t}$$

If K is constant in space and the material is **Orthotropic**, the 3D Heat Conduction Eq is:

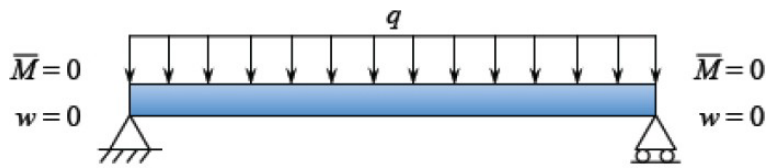
$$K_x (\partial^2 T / \partial x^2) + K_y (\partial^2 T / \partial y^2) + K_z (\partial^2 T / \partial z^2) + (e_{\text{gen}}) = \rho c (\partial T / \partial t)$$

The above Eqs are **Linear** since for each term of the Eq, the Field/Dependent Variable (i.e., T) is to the **first power**.

If the Thermal Conductivity components are functions of Temperature, then the above equation becomes **Nonlinear** since it will have the Field/Dependent Variable (i.e., T) to the **second power** for each term of the Eq. as:

$$K_x(T) (\partial^2 T / \partial x^2) + K_y(T) (\partial^2 T / \partial y^2) + K_z(T) (\partial^2 T / \partial z^2) + (e_{\text{gen}}) = \rho c (\partial T / \partial t)$$

Governing Eq for Bending of a Beam:

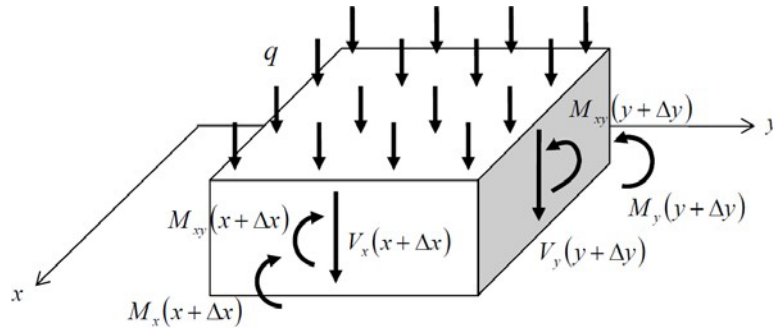


$$EI \frac{d^4 w}{dx^4} = q(x)$$

$$\begin{aligned}\frac{d^3w}{dx^3} &= \frac{qx}{EA} + C_1 \\ \frac{d^2w}{dx^2} &= \frac{qx^2}{EA2} + C_1x + C_2 \\ \frac{dw}{dx} &= \frac{qx^3}{EA6} + \frac{C_1x^2}{2} + C_2x + C_3 \\ \frac{dw}{dx} &= \frac{qx^4}{EA24} + \frac{C_1x^3}{6} + \frac{C_2x^2}{2} + C_3x + C_4\end{aligned}$$

$$w(x) = \frac{qx}{24EA}(l^3 - 2lx^2 + x^3)$$

Governing Eq for Bending of a Plate:



$$\boxed{\frac{\partial^4 w}{\partial x^4} + 2\frac{\partial^4 w}{\partial x^2 \partial y^2} + \frac{\partial^4 w}{\partial y^4} = -\frac{q}{D}}$$

Where:

$$D = \frac{Eh^3}{12(1-\nu^2)}$$

V. Finite Element Modeling & Analysis:

Any Finite Element Modeling (FEM) and Finite Element Analysis (FEA) includes the following three main steps: **(1) Pre-Processing, (2) Model Solution, (3) Post-Processing.**

V.1. Step 1: Pre-Processing

1. : V.1.1. Object Generation:

1. 3D: **3D Solid Modeling with Solid Elements**
2. 1D & 2D: **1D Beam Element & 2D Shell Elements**

2. : V.1.2. Pick Analysis Type (for Step 2: Model Solution):

1. **Structural Analysis (Static or Dynamic; Linear or Non-Linear)**
2. **Heat Transfer Analysis (Steady State or Transient; Linear or Non-Linear)**
3. **Fluids Analysis (Steady State or Transient; Linear or Non-Linear)**

3. : V.1.3. Meshing (see Meshing details):

1. **Select Elements**
2. **Select Nodes**

4. : V.1.4. Material Properties (see [Cij] details):

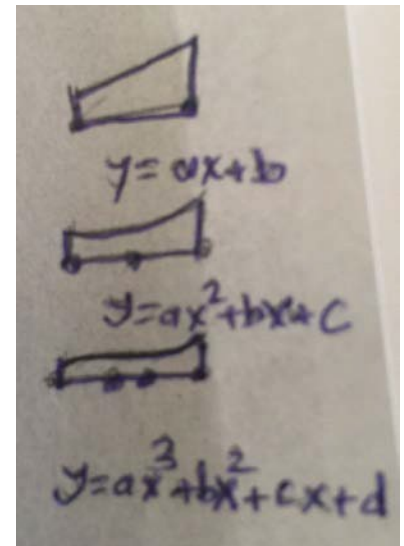
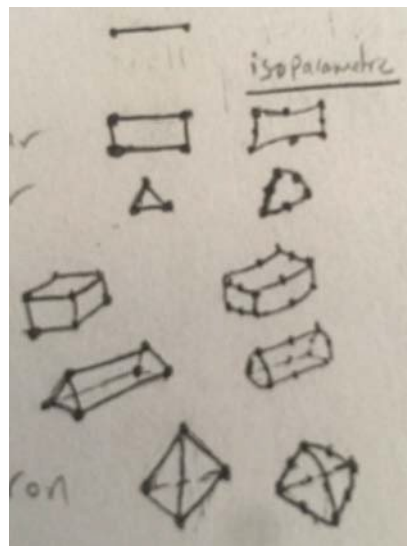
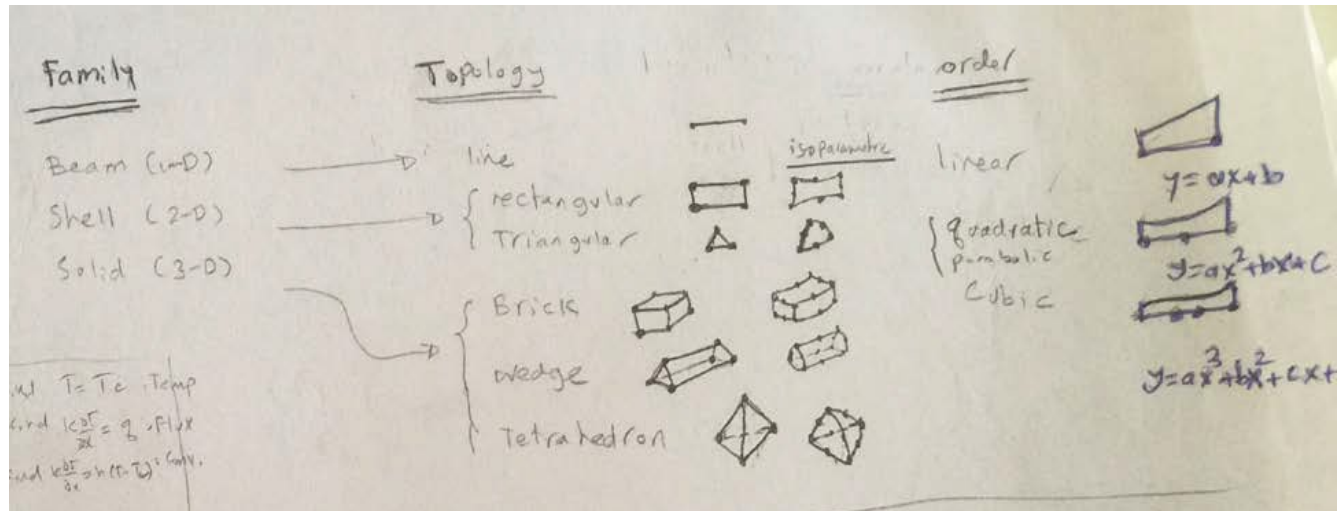
1. **Isotropic (Metals/Polymers/Ceramics)**
2. **Transversely Isotropic (Composite Plies)**
3. **Orthotropic (Smart Materials, Superconducting, Thin Films, etc)**
4. **Anisotropic (Often due to rotation of Trans. Iso. Or Orthotropic materials)**

5. : V.1.5. Boundary Conditions (see BCs details):

1. **Applied Loads**
2. **Restraints**

V.1.3. MESHING Details (Elements & Nodes)

SUMMARY OF MESHING Details (Elements & Nodes)



Family

Topology

Order

Beam (1-D)

Line

Linear

Shell (2-D)

Rectangular
Triangular

Quadratic
Parabolic

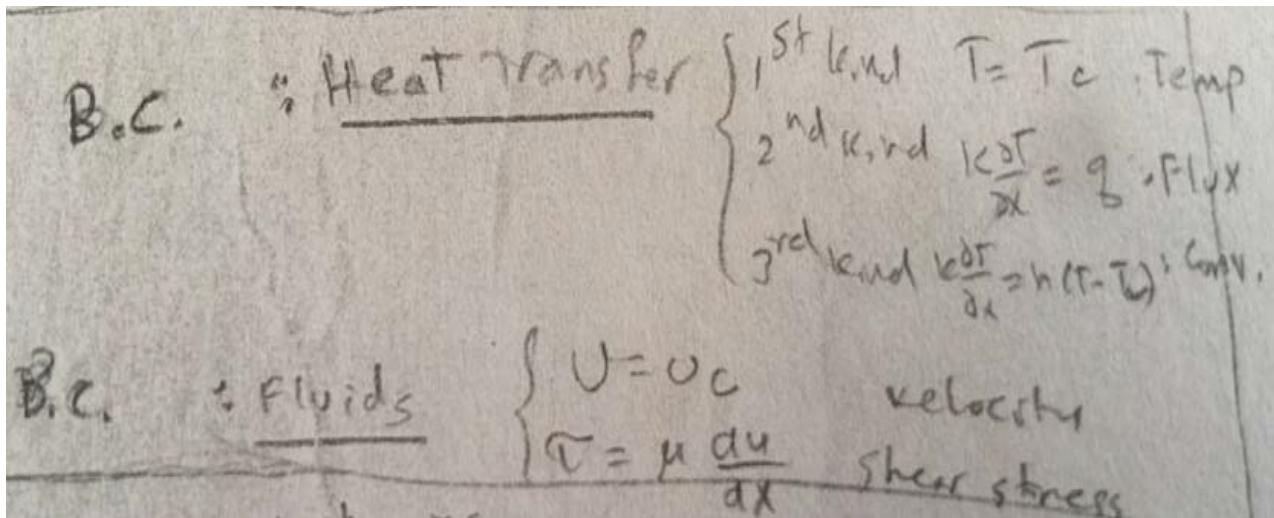
Solid (3-D)

Brick
Wedge
Tetrahedron

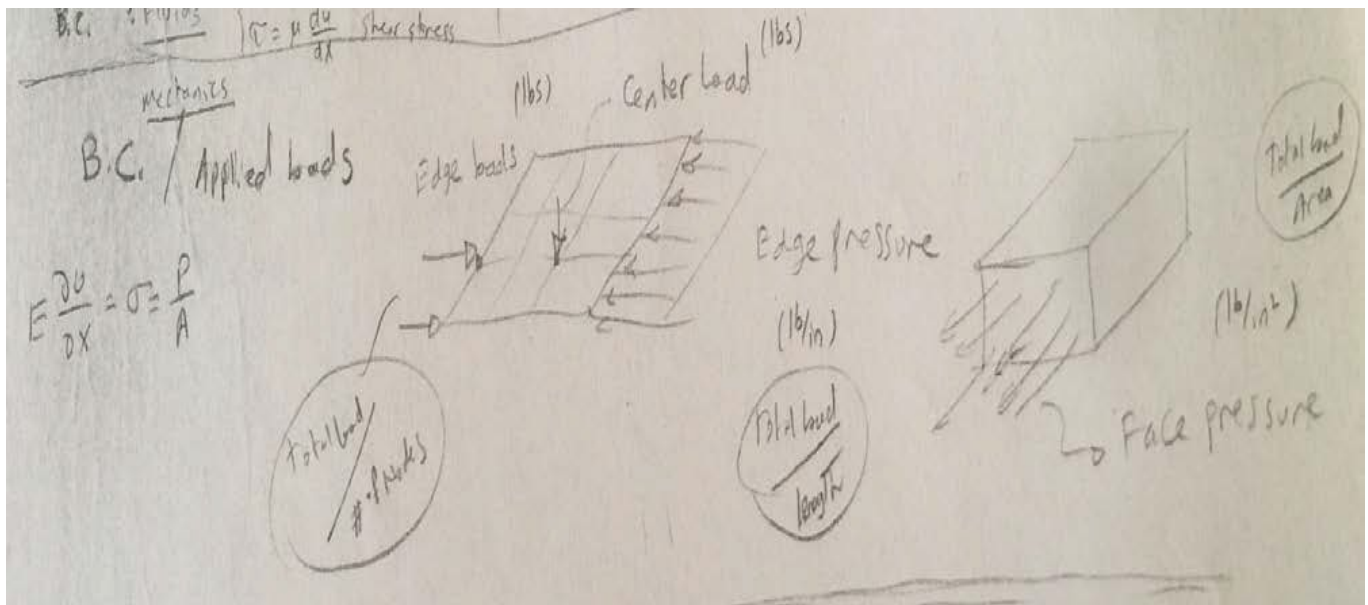
Cubic
Cubic
Cubic

V.1.5. Boundary Conditions Details

V.1.5.1. Boundary Conditions for Heat Transfer (HT) & Fluids (F)



V.1.5.2. BCs: Applied Loads in Mechanics



V.1.5. Boundary Conditions Details

V.1.5.3. BCs: Restraints in Mechanics

(U)		Trans X, Y, Z	Rotation X, Y, Z	6-Dof's	Legend $\begin{cases} \text{Fix} = 0 \\ \text{Free} = f \end{cases}$	
B.C. / Restraints					Trans X, Y, Z	Rot X, Y, Z
Clamp :		fixes all 6-Dof's		→	0, 0, 0	0, 0, 0
Ball-joint :		Trans Fixed Rots Free		→	0, 0, 0	f, f, f
Pin-joint :		All Dof's Fixed except one Rot.		→	0, 0, 0	0, 0, f
Slider :		All Dof's Fixed except $\begin{cases} \text{two Trans.} \\ \text{one Rot.} \end{cases}$		→	f, 0, f	0, f, 0
Roller :		All Dof's Fixed except $\begin{cases} \text{one Trans.} \\ \text{one Rot.} \end{cases}$		→	f, 0, 0	0, 0, f
Plane of Symmetry :		All Dof's Fixed except $\begin{cases} \text{one Trans.} \\ \text{one Rot.} \end{cases}$		→	0, f, f	f, 0, 0

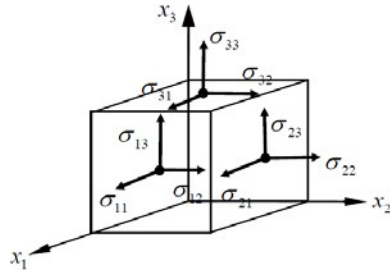
V. Finite Element Modeling & Analysis:

V.2. Step 2: Model Solution

V.2.1: Solve/Run

V.3. Step 3: Post-Processing

V.3: Output Selection (e.g., in Mechanics):



$$[\sigma_{ij}] = \begin{bmatrix} \sigma_{11} & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} \end{bmatrix}$$

$$\sigma_{ij} = \sigma_{ji}$$

$$\{\sigma_i\}_{6 \times 1} = [C_{ij}]_{6 \times 6} \{\epsilon_i\}_{6 \times 1}$$

V.3.1. Stresses (6 Independent Components, of a 2nd Order Tensor)

X-Normal (σ_{xx})	XY-Shear (σ_{xy})
Y-Normal (σ_{yy})	YZ-Shear (σ_{yz})
Z-Normal (σ_{zz})	ZX-Shear (σ_{zx})

V.3.2. Strain (6 Independent Components, of a 2nd Order Tensor)

X-Normal (ϵ_{xx})	XY-Shear (ϵ_{xy})
Y-Normal (ϵ_{yy})	YZ-Shear (ϵ_{yz})
Z-Normal (ϵ_{zz})	ZX-Shear (ϵ_{zx})

V.3.3. Displacements (3 Independent Components, of a Vector = 1st Order Tensor)

X-Direction (U_x)
Y-Direction (U_y)
Z-Direction (U_z)

V.3.4. Von Mises Stresses (Distorsion Energy Theory)

VM Stress in terms of principle stresses

$$\sigma_{VM} = \sqrt{\frac{\sigma_1 - \sigma_2}{2}^2 + \frac{\sigma_2 - \sigma_3}{2}^2 + \frac{\sigma_3 - \sigma_1}{2}^2}$$

VM Stress in terms of 6 components actual stresses

$$\sigma_{VM} = \sqrt{\frac{(\sigma_{xx} - \sigma_{yy})^2 + (\sigma_{yy} - \sigma_{zz})^2 + (\sigma_{zz} - \sigma_{xx})^2 + 6(\tau_{xy}^2 + \tau_{yz}^2 + \tau_{zx}^2)}{2}}$$

VI: Failure Analysis & Factor of Safety (FOS):

Two Considerations:

- (I) Stress Critical Structures
- (II) Stiffness (Deflection/Displacement) Critical Structures
- (III) Materials Selection Issues Related to Failure Analysis

VI.I. Stress Critical Structures: $FOS = \sigma_{\text{yield}} / \sigma_{\text{max}} > 1$

VI.I.1. For Isotropic Materials (Metals, Polymers, Ceramics)

Use Maximum Von Mises Stress:

$$FOS = (\sigma_{\text{yield}} / \sigma_{\text{max Von Mises}}) > 1$$

VI.I.2. For Anisotropic Materials (Composite Materials)

Use Maximum Stress Criterion: i.e., find FOS for each stress-component (i.e., $FOS = \text{direction-strength}/\text{maximum direction-stress}$), and then the Min FOSs is the FOS.

For example:

(1) Find FOSs for Normal Stresses (3 componenets):

FOS for σ_{xx} : $FOS |_{\sigma_{xx}} = (\text{Normal Strength in X-Direction} / \sigma_{xx \text{ max}}) > 1$

Likewise for σ_{yy} & σ_{zz} : $FOS |_{\sigma_{yy}}$ & $FOS |_{\sigma_{zz}} > 1$

(2) Find FOSs for Shear Stresses (3 independent componenets): FOS for σ_{xy} :

$FOS |_{\sigma_{xy}} = (\text{Shear Strength in XY-Plane} / \sigma_{xy \text{ max}}) > 1$ Likewise for σ_{yz} & σ_{zx}

: $FOS |_{\sigma_{yz}}$ & $FOS |_{\sigma_{zx}} > 1$

(3) Final FOS = Min ($FOS |_{\sigma_{xx}}$, $FOS |_{\sigma_{yy}}$, $FOS |_{\sigma_{zz}}$, $FOS |_{\sigma_{xy}}$, $FOS |_{\sigma_{yz}}$, $FOS |_{\sigma_{zx}}$) > 1

Typical FOS in Design = 1.5, Human Life Involved= 2.5 (if confident about your assumption, modeling, and analysis); otherwise pick Larger (up to 4).

In Space Applications since weight is a critical factor, the analyses are conducted accurately, and hence it is afforded to use lower FOSs = ~ 1.1 to 1.25.

VI.I.3. Remedies if $FOS = \sigma_{yield} / \sigma_{max} < 1$:

We want to increase FOS to > 1 : So,

(1) Either Increase the Numerator (i.e., σ_{yield}) \uparrow

(2) Or Decrease the Denominator (i.e., σ_{max}) \downarrow

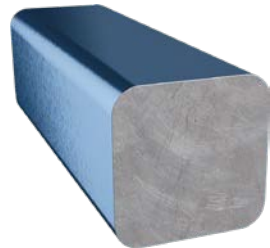
(1) Increase the Numerator (i.e., σ_{yield}) \uparrow :

By changing the material to a **Stronger Material** with higher σ_{yield} ,
e.g., change Aluminum to Steel.

(2) Decrease the Denominator (i.e., σ_{max}) \downarrow :

σ can, in general, be modeled as $\sigma = F / A$

$$\sigma \downarrow = F \downarrow / A \uparrow$$



Since F is a design parameter and often cannot be changed, hence the remedy is to use a “beefier” components, i.e., with higher cross-sectional area, A , where stress is acting on.

For example: The beam with cross-section of b =width, h =thickness, and l =length

For **Axial Loading** (*Tension* or *Compression*): $\sigma \downarrow = F / A \uparrow$, Increase $A = b \times h$

For **Lateral Loading** (*Bending*): $\sigma \downarrow = MC/I \uparrow$ & $I \uparrow = b h^3/12$,

Increase h since it has cubic effect in increasing I than b .

VI.II Stiffness (Deflection/Displacement) Critical Structures: Deflection/Displacement < δ_{limit}

VI.II.1. Limites of Deflections/Displacement in Designs:

For those structures that have $FOS \gg 1$, then next consideration is if the material is stiff enough to give **Deflection/Displacement < δ_{limit}** (say, not more than 10% of the thickness, to also stay within Linear Regime; e.g., assuming the nominal cross-section is 10mm x 10mm, then the $\delta_{limit} = 1$ mm or less)

Therefore, in many designs various components have their own Critical Deflection/Displacement Limits (δ_{limit}) that designers should check with the sponsor/client; however, a rule-of-thumb is that:

**Deflection/Displacement < 1 mm=0.040" (for a nominal cross-section:
10mm x 10mm to also stay within the Linear Regime)**

VI.II.2. Remedies if Deflection/Displacement > δ_{limit} :

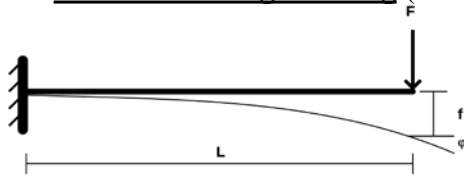
The amount of **Deflection/Displacement** in a structure is a function of the structure Stiffness (i.e., **Young's Modulus, E** : $\sigma = E \epsilon$).

For Axial Loading (*Tension* or *Compression*): $\sigma = F / A = E \epsilon = E \Delta L / L$:

$$F = EA \Delta L / L \implies \Delta L = \delta = F L / EA$$

Then, if F , L , and A are the design parameters that you cannot change, then the only remedy is to use a Stiffer Materials, e.g., change Aluminum to Steel

For Lateral Bending Loading (*Bending*):



$$f = \delta = F L^3 / 3 E I, \text{ where } I = b h^3 / 12$$

Understanding that L has a cubic effect on δ compared with F ; however, they both may be the design parameters that you cannot change. In addition, EI is called “**Flexural Rigidity**” that should increase in this case. Again, note that to increase I , h has cubic effect in increasing I than b . Finally, to reduce δ , E (i.e., the materials stiffness) should increase, e.g., change Aluminum to Steel.

VI.III. Materials Selection Issues Related to Failure Analysis

If Weight is a Major Factor in Design, then:

VI.III.1. For Stress-Critical Structures:

For Stress-Critical Structures the parameter that should be maximized is not only σ_{yield} (i.e., *Yield Strength*), but “ σ_{yield} / ρ ” (i.e., *Specific Strength*) should be maximized, where ρ is the density of the material.

For example: (σ_{yield} / ρ) unit in the following is: “KN*m/Kg”

$$(\sigma_{yield} / \rho)|_{\text{for C/E Composites}} = \sim 2,000 \gg (\sigma_{yield} / \rho)|_{\text{Steel}} = \sim 200 > (\sigma_{yield} / \rho)|_{\text{Al}} = \sim 100$$

VI.III.2. For Stiffness-Critical Structures:

For Stiffness-Critical Structures the parameter that should be maximized is not only E (i.e., *Young's Modulus or Stiffness*), but “ E / ρ ” (i.e., *Specific Stiffness*), should be maximized, where ρ is the density of the material.

For example: (E / ρ) unit is in “ $m^2 s^{-2} * 10^{-6}$ ”.-

$$(E / \rho)|_{\text{for C/E Composites}} = \sim 100 \gg (E / \rho)|_{\text{Steel}} = \sim 25 = (E / \rho)|_{\text{Al}} = \sim 25$$